SEISMIC VULNERABILITY ASSESSMENT OF RC STRUCTURES USING INCREMENTAL DYNAMIC ANALYSIS



by

Uzma Iqbal

(NUST201260989MSCEE15212F)

A thesis submitted in partial fulfillment of the requirements for the degree of

Master of Science

in

Structural Engineering

NUST Institute of Civil Engineering (NICE)
School of Civil and Environmental Engineering (SCEE)

This is to certify that the thesis titled

SEISMIC VULNERABILITY OF RC STRUCTRE USING INCREMENTAL DYNAMIC ANALYSIS

submitted by

Uzma Iqbal

has been accepted towards the partial fulfillment of the requirements for the degree

of

Master of Science in Structural Engineering

Dr. Hassan Farooq

Associate Professor

School of Civil and Environmental Engineering (SCEE)

National University of Sciences and Technology (NUST), Islamabad, Pakistan

DEDICATION TAKES A LIFETIME; BUT DREAMS ONLY LAST FOR A NIGHT.

ACKNOWLEDGEMENTS

I am thankful to Almighty Allah, who gave me strength and patience to complete my research. I would like to thank my advisor Dr. Hassan Farooq, for his faith and guidance during this research. I am also extremely grateful to the committee members, for their sincere guidance to complete my research work.

I pay my earnest gratitude to Mr and Mrs Iqbal Ahmad, Mr and Mrs Ali and Mr Shahzad Ahmad for the continuous support and encouragement.

Table of Contents

Acknowledgement	vi
4bstract	<i>xi</i>
Table of Contents	
List of Figures	
List of Tables	x
Chapter - 1	1
1.1.Introduction	1
1.2.Aims and Objectives	/
1.2.Amis and Objectives	······································
Chapter - 2	6
2.1.Introduction	<i>6</i>
2.2.Background	6
2.3.Methods of Vulnerability Assessment	6
2.3.1. Empirical Method	
2.3.2. Judgment based Method	11
2.3.3. Analytical assessment method	13
2.4.Methods of Vulnerability Assessment	6
2.4.1.Beam Column Joint Element	27
2.4.1.2.Joint behavior under Seismic Forces	27
2.4.1.3.Background and Literature Review	30
2.4.1.4.Lauara Lowes Joint Element	31
2.4.1 Model Evaluation and Verification	35

Chapter - 3	6
3.1 Introduction	6
3.2 Background	6
3.3 Ground motion Database	6
3.3.1.Introduction.	41
3.3 Criteria for Selection of Ground motions	41
3.4.1 Source Magnitude	42
3.4.3 Site-Source	42
3.4.4 Number of Records per Event	42
3.4.5 Strong Ground motion	42
3.4.6 Strong Ground motion Instruments Capability	42
3.4.7 Instruments Location.	43
3.5 Ground motion Selection Procedure	4 3
3.5.1 Design Spectrum	43
3.5.2 Specifying Criterion	44
3.6 Spectrum Matching for the selection of Earthquake	
3.6.1 Spectrum matching for range of period	
3.6.2. Spectrum matching for single period	
Chapter - 4	62
4.1.Introduction	62
4.2.Introduction to OpenSEES	62
4.3.Why OpenSEES	62
4.4.Calibration of Analytical tool	62
4.5.1. Introduction to Saclay frame	63

.4.5.1.1. Characteristic of model	63
4.5.1.2 Material properties	66
4.5.1.3. Observed damages	65
4.5.2. Modeling in OpenSees	66
4.6. Structural Response	71
1.7. Brief Introduction to the Subject Building	74
1.8. Methods of performing IDA	75
Chapter - 5	77
5.1. Introduction	77
5.2.Introduction to the Structure	77
5.4. Near Field Ground Motion for Rang of period	77
5.5.Far Field Ground Motion for Rang of Period	79
5.6. Near Field Ground Motion for Single of period	79
5.7.Far Field Ground Motions for Single Period	82
Fragility Curve	87
Chapter - 6	88
Referncess	89

List of Figures

Figure 2.1: Continous Vunerability Function	9
Figure 2.2: Continous Vunerability Function	10
Figure 2.3: Mean Damage Ratio with MMI	12
Figure 2.4: Frame Work for Analytical Vulnerability	13
Figure 2.5: . Shinghal and Kinremidjan Fragility Curve	15
Figure 2.6: Damage Distribution	16
Figure 2.7: Building Joint in OpenSEES	26
Figure 2.8: Internal Forces in Joint Perimeter due to Cyclic Loading	26
Figure 2.9: Internal Forces acting on Joint Core due to Cyclic Loading	27
Figure 2.10: Typical Failure due Bond slip, Bar pullout and Shear Deformation	28
Figure 2.11: Laura Lowes Joint Element	29
Figure 2.12: Load deformation curve for Loading and re-loading	30
Figure 2.13: Bar Slip Stress	32
Figure 2.14: Experimental Model for Beam Column Joint	34
Figure 2.15: Experimental Results of Beam Column Joint	35
Figure 2.16: Analytical Results	36
Figure 3.1: Design Spectrum	41
Figure 3.2: Far Field Ground Motions	43
Figure 3.3: Group 1 Far Field Ground Motions	43
Figure 3.4: Group 2 Far field Ground Motions	45
Figure 3.5: Group 3 Far Field Ground Motions	46
Figure 3.6: Group 4 Far Field Ground Motions	46
Figure 3.7: Near Field Ground Motions	47
Figure 3.8: Group 1 Near Field Ground Motions	49
Figure 3.9: Group 2 Near Field Ground Motions	49
Figure 3.10: Group 3 Near Field Ground Motions	50
Figure 3.11: Group 4 Far Field Ground Motions	50
Figure 3.12: Far Field Ground Motions	51

Figure 3.13: Group 1 Ground Motion for Near Field	. 53
Figure 3.14: Group 2 Far Field Ground Motions	. 53
Figure 3.15: Group 3 Far Field Ground Motions	. 54
Figure 3.16: Group 4 Far Field Ground Motions	. 54
Figure 3.17: Near Field Ground Motions	. 55
Figure 3.18: Group 1 Ground Motion for Near Field	. 57
Figure 3.19: Group 2 Far Field Ground Motion	. 57
Figure 3.20: Group 3 Far Field Ground Motion	. 58
Figure 3.21: Group 4 Far Field Ground Motion	. 58
Figure 4.1: Two Storey Full Model of Saclay Frame	. 61
Figure 4.2: Deatils of Frame Reinforcement (Chaudat et al., 2005)	. 63
Figure 4.3: Stress Strain Model for Concrete	. 66
Figure 4.4: Stress Strain Model for Steel	. 66
Figure 4.5: Distribution of Masses on Nodes	. 67
Figure 4.6: Response of Seclay Frame at 0.05g PGA	. 69
Figure 4.7: Response of Seclay Frame at 0.1g PGA.	. 70
Figure 4.8: Response of Seclay Frame at 0.2g PGA.	. 71
Figure 4.9: Response of Seclay Frame at 0.3g PGA.	. 71
Figure 4.10: Response of Seclay Frame at 0.4g PGA	. 72
Figure 4.11: Elevation of Subject Portal Frame.	. 73
Figure 5.1: IDA Curves for Near Fields Ground Motion	. 76
Figure 5.2: 16%, 50% and 84% Fractiles for IDA.	. 77
Figure 5.3: IDA Curves for Far Field Ground Motion	. 79
Figure 5.4: 16%, 50% and 85% Fractile for IDA	. 81
Figure 5.5: IDA Curves for Near Field Ground Motion	. 82
Figure 5.6: 16%, 50% and 84% Fractile for IDA	. 83
Figure 5.7: IDA Curves for Far Field Ground Motion	. 85

List of Tables

Table 1.1: Damage Probability Matrix	8
Table 1.2: Judgement Based Vulnerability Function	11
Table 3.1: Far Field Ground Motions	44
Table 3.2: Near Field Ground Motions	48
Table 3.3: Far Field Ground Motion	52
Table 3.4: Near Field Ground Motions	56
Table 4.1: Distribution of Masses at Different Nodes	68
Table 4.2: Damping Coefficient	69
Table 5.1: Performance Based Limits for Near Field Ground Motions	77
Table 5.2: Performance Based Limits for Far Field Ground Motions	80
Table 5.3: Performance Based Limits for Near Field Ground Motions	83
Table 5.4: Performance Based Limits for Far Field Ground Motions	85

Abstract

In most of the developing countries, where major portion of the existing building stock belongs to the era when seismic codes were not implemented, a thorough seismic vulnerability assessment is necessary. As the representative building which is considered in this study is typical building belongs to Pakistan. Poor detailing, low strength materials and low standard construction leads brittle failures in such structures. As with the evolution of performance based earthquake engineering it is necessary to estimate the fragility of such buildings to assess and analyze according to the new limit states.

The main purpose of this study is to evaluate the effect of selection and scaling of the earthquake records on the IDA curves results. The subject building which is gravity design building, its time period is determined and based site specific spectrum from UBC for region 2B. Two methods are chosen one is selection on the basis of single period and other is on the basis of range of period for the same building. The ground motion records are based on the local region fall in the category of moderate earthquake region. If the high seismic region was chosen, the bias in the dynamic structural instability would occur.

Incremental dynamic analysis is performed using a powerful tool OpenSEES. This software is validated using the experimental results obtained by a full scale frame which is tested on shake table with increasing intensity. Then fractiles from those IDA curves are found and performance limits are marked.

For the Near Field ground motion which are selected for the period of range, IDA curve don't scattered and give reliable results for the performance limits. When far field motions for the same method are used, the IDAs tend to be not reliable. As the selection of earthquakes are more likely from the same site but different station. When selection of earthquakes is done based on single period a large diversity is observed. Then fragility curves are drawn based on performance based limits.