Combine UAV and 2D ERT Approach for Identification of Water Leakage Zones in Gul Dheri Dam, Akora Khattak, Nowshera



By

Capt Irfan Haider Khan (Leader)	CMS-356439
Capt Hamza Shahid	CMS-356446
Capt Abdullah Khan	CMS-356449
Capt Ejaz Akram	CMS-356450

Military College of Engineering, Risalpur National University of Sciences and Technology Islamabad, Pakistan (2024)

Combine UAV and 2D ERT Approach for Identification of Water Leakage Zones in Gul Dheri Dam, Akora Khattak, Nowshera



By

Capt Irfan Haider Khan (Leader)	CMS-356439
Capt Hamza Shahid	CMS-356446
Capt Abdullah Khan	CMS-356449
Capt Ejaz Akram	CMS-356450

A thesis submitted to the National University of Sciences and Technology, Islamabad,

in partial fulfillment of the requirements for the degree of

Bachelors in Civil Engineering

Supervisor: Dr. Muhammad Junaid

Military College of Engineering, Risalpur National University of Sciences and Technology Islamabad, Pakistan (2024)

THESIS ACCEPTANCE CERTIFICATE

Certified that final copy of UG Thesis written by Mr <u>Irfan Haider Khan</u> (Registration No. <u>356439</u>), of <u>Military College of Engineering, NUST, Risalpur</u> has been vetted by undersigned, found complete in all respects as per NUST Statutes/ Regulations/ UG Policy, is free of plagiarism, errors, and mistakes and is accepted as partial fulfillment for award of UG degree. It is further certified that necessary amendments as pointed out by GEC members and foreign/ local evaluators of the scholar have also been incorporated in the said thesis.



Signature:

Name of Supervisor: Dr Muhammad Junaid

Date: 15/05/2024

CERTIFICATE OF APPROVAL

This is to certify that the research work presented in this thesis, entitled "Combine UAV and 2D ERT Approach for Identification of Water Leakage Zones in Gul Dheri Dam, Akora Khattak, Nowshera" was conducted by Mr. Irfan Haider Khan under the supervision of Dr Muhammad Junaid. No part of this thesis has been submitted anywhere else for any other degree. This thesis is submitted to the Military College of Engineering, NUST, Risalpur in partial fulfillment of the requirements for the degree of Bachelors of Civil Engineering.

Student Name: Irfan Haider Khan

Signature:



Supervisor Name: Dr Muhammad Junaid

AUTHOR'S DECLARATION

I, <u>Capt Irfan Haider Khan</u> hereby state that my UG thesis titled "Combine UAV and 2D ERT Approach for Identification of Water Leakage Zones in Gul Dheri Dam, Akora Khattak, Nowshera" is my own work and has not been submitted previously by me for taking any degree from National University of Sciences and Technology, Islamabad or anywhere else in the country / world.

At any time if my statement is found to be incorrect even after I graduate, the university has the right to withdraw my UG degree.



Student Name: Irfan Haider Khan

Signature:

Date: 15/05/2024

PLAGIARISM UNDERTAKING

I solemnly declare that research work presented in the thesis titled "Combine UAV and 2D ERT Approach for Identification of Water Leakage Zones in Gul Dheri Dam, Akora Khattak, Nowshera" is solely my research work with no significant contribution from any other person. Small contribution / help wherever taken has been duly acknowledged and that complete thesis has been written by me.

I understand the zero tolerance policy of the HEC and National University of Sciences and Technology (NUST), Islamabad towards plagiarism. Therefore, I as an author of the above titled thesis declare that no portion of my thesis has been plagiarized and any material used as reference is properly referred/cited.

I undertake that if I am found guilty of any formal plagiarism in the above titled thesis even after award of UG degree, the University reserves the rights to withdraw/revoke my UG degree and that HEC and NUST, Islamabad has the right to publish my name on the HEC/University website on which names of students are placed who submitted plagiarized thesis.



Signature:

Name: Capt Irfan Haider Khan

ACKNOWLEDGEMENTS

Starting with the name of Allah, the most compassionate, the most merciful. And may prayers and peace be upon our Holy Prophet Muhammad (S.A.W), His servant and messenger.

Firstly we are thankful to Allah Almighty for guiding us from the start to end of this project. Then, we would like to express our sincere appreciation to the individuals who have supported and contributed to the completion of this research paper.

First and foremost, we extend our deepest gratitude to our Advisor, Dr. Muhammad Junaid, whose guidance, expertise, and unwavering support were invaluable throughout the research process. His insightful feedback and encouragement significantly shaped the direction and quality of this work.

We are thankful to the members of Soil Laboratory of our college for their valuable discussions and assistance.

We would like to acknowledge the contributions of our friend, Engr Furqan, for his collaboration and input, which enhanced the depth and breadth of this study.

We are grateful to our family for their unwavering encouragement, understanding, and patience during the demanding phases of this research endeavor. Their support has been a constant source of strength.

Lastly, I extend my thanks to all those who, directly or indirectly, contributed to this research, even if their names are not explicitly mentioned. Your support and encouragement have been invaluable.

Thank you!

TABLE OF CONTENTS

DECLARATION Error! Bookman	k not defined.
ACKNOWLEDGEMENTS	6
ABSTRACT	12
TABLE OF CONTENTS	7
TABLE OF FIGURES	9
LIST OF TABLES	
CHAPTER 1	12
INTRODUCTION	13
1.1 Background Study	13
1.1.1 Importance of Resistivity Testing in Dam Safety	13
1.1.2 Background Study of 2D-Electrical Resistivity Tomography (ERT)	14
1.1.3 Methodology of Electro Tomography for Resistivity Testing	15
1.1.3 Using 2D Electrical Resistivity Tomography (ERT) offers several benefit various fields of application	s across 15
1.2 Problem Statement	
1.3 Objectives	17
1.3.1 Objective 1	
1.3.2 Objective 2	
1.3.4 Objective 4	
1.4 Organization of the Report	
CHAPTER 2	
LITERATURE REVIEW	
2.1 Introduction	
2.2 Dam Health Assessment	
2.3 Teton Dam Disaster 1976	
2.4 Various Techniques of Dam Health Assessment	20
2.4.1 Geotechnical Investigations.	20
2.4.2 Instrumentation and Monitoring.	20
2.4.3 Seismic Analysis.	20
2.4.4 Non-Destructive Testing (NDT).	20
2.5 Electric Resistivity Tomography	21
2.5.1 Procedure.	21
2.5.2 Benefits.	21

2.6 Surveying and Geographic information System (GIS): Exploring the capabilities of Unmanned Aerial Vehicle (UAV/Drones)	22
2.6.1 General	22
2.6.2 Benefits of Drone Survey	22
2.6.3 Methodology	23
2.6.4 Integration with GIS	23
2.7 Summary	23
CHAPTER 3	25
METHODOLOGY AND FIELD TESTS / RESULTS	25
3.1 ERT Survey	25
3.1.1 General	25
3.1.2 Resisitivity Imaging Equipment	26
3.1.3 Fieldwork / Results	26
3.1.4 Areas of zones in ERT Profile using ArcGIS	42
3.1.5 Dam Site View with zones of all profiles	50
3.2 Drone Survey	51
3.2.1 Benefits of Drone Survey	51
3.2.2 Results of Drone Survey	52
CHAPTER 4	54
REMEDIAL MEASURES	54
References	57

TABLE OF FIGURES

Figure 1:Teton Dam Failure	
Figure 4: ERT Profiles	27
Figure 5:ERT P-1	27
Figure 6: ERT P-1 (2D-ERT)	
Figure 7: ERT P-1 (2D-ERT)	
Figure 8: ERT P-2	30
Figure 9: ERT P-2 (2D-ERT)	
Figure 10: ERT P-2 (2D-ERT)	
Figure 11: ERT P-3	
Figure 12: ERT P-3 (2D-ERT)	
Figure 13: ERT P-3 (2D-ERT)	
Figure 14: ERT P-4	
Figure 15: ERT P-4 (2D ERT)	
Figure 16: ERT P-4 (2D ERT)	
Figure 17: ERT P-5	
Figure 18: ERT P-5 (2D ERT)	
Figure 19: ERT P-5 (2D ERT)	
Figure 20: ERT P-6	
Figure 21: ERT P-6 (2D ERT)	40
Figure 22: ERT P-6 (2D ERT)	40
Figure 23: Georeferencing 1	
Figure 24: Georeferencing 2	
Figure 25: Classification 1	43
Figure 26: Classification 2	43
Figure 27: ERT P-1 (Classified)	44
Figure 28: ERT P-2 (Classified)	45
Figure 29: ERT P-3 (Classified)	46
Figure 30: ERT P-4 (Classified)	47
Figure 31: ERT P-5 (Classified)	
Figure 32: ERT P-6 (Classified)	49
Figure 33: Dam view with all profiles	
Figure 34: Drone Survey	51
Figure 35: Aerial View of Dam Site	
Figure 36: Digital Elevation Model	
Figure 37: Camera Locations	53

LIST OF TABLES

Table 1: AREAS (ERT P-1)	44
Table 2: AREAS (ERT P-2)	45
Table 3: AREAS (ERT P-3)	46
Table 4: AREAS (ERT P-4)	47
Table 5: AREAS (ERT P-5)	48
Table 6: AREAS (ERT P-6)	49
Table 7: Drone Survey Specifications	53
Table 8: Remedial Measures	56

LIST OF ACRONYMS

ERT	-	Electrical Resistivity Tomography
NDT	-	Non-Destructive Testing
UAV	-	Unarmed Aerial Vehicle
GIS	-	Geographic Information System
GCP	-	Ground Control Point

ABSTRACT

This research explores the fusion of 2D Electrical Resistivity Tomography (ERT) and advanced Drone Survey techniques to identify and map potential leakage zones within dam structures. Addressing the critical need for comprehensive and non-invasive methodologies in dam safety assessment, this study aims to revolutionize the identification and characterization of subsurface anomalies indicative of potential leaks.

The integration of 2D ERT, a geophysical method measuring subsurface resistivity variations, and Drone Survey technology, offering high-resolution aerial imaging capabilities, provides a synergistic approach. The ERT technique enables the characterization of subsurface resistivity distribution, while drone-based surveys provide detailed visual data of surface features, facilitating a holistic understanding of the dam's condition.

Through a series of field experiments conducted at Gul Dheri Dam, multi-electrode ERT surveys were conducted to acquire resistivity data, capturing the subsurface variations. Simultaneously, drone-mounted cameras captured high-resolution imagery of the dam surface and surrounding areas, allowing for detailed visual inspections.

Data integration and analysis involved use of Zone Resz 2D software for creating 2D cross sectional subsurface profiles and ArcGIS for calculation of areas of zones. Interpretation of the integrated data facilitated the identification of anomalous zones that may indicate potential seepage or leakage paths within the dam structure.

This research not only contributes to the advancement of geophysical and remote sensing methodologies but also holds significant implications for dam safety management practices. The outcomes offer insights into the development of efficient monitoring strategies to mitigate risks associated with potential leaks in dams, thereby safeguarding communities and infrastructure.

Keywords: Data interpretation, Electrical Resistivity Tomography, Drone Survey, leakage zones, mapping, ArcGIS, subsurface evaluation, area calculation, dam health assessment.

CHAPTER 1

INTRODUCTION

1.1 Background Study

Electrical Resistivity Tomography (ERT) is an advanced geophysics method used to determine sub surface's resistivity distribution by making measurements on ground surface. ERT data is rapidly collected with an automated multi-electrode resistivity meter. Electro tomography is a non-invasive geophysical method used to image the subsurface of earth structures, like dams.

1.1.1 Importance of Resistivity Testing in Dam Safety

1.1.1.1 Early Detection of Leaks. Resistivity testing helps in detecting leaks and seepage in the dam structure. ERT can be a valuable tool for dam safety assessments, particularly in the early detection of leaks. By providing detailed subsurface information and detecting changes in resistivity associated with water infiltration, ERT helps identify and address potential issues before they escalate into major problems

1.1.1.2 Identifying Weak Zones. It assists in identifying areas of low resistivity, indicating potential weak zones in the dam.

1.1.1.3 Monitoring Structural Stability. Regular testing ensures ongoing monitoring of the dam's structural stability and integrity.

1.1.1.4 Preventing Catastrophic Failure. Early identification of potential issues helps in preventing catastrophic dam failures and ensures public safety. By leveraging the capabilities of ERT for subsurface imaging and monitoring, dam operators and engineers can enhance their ability to detect early warning signs of potential failures, implement timely interventions, and ultimately reduce the risk of catastrophic dam collapses or structural failures.

1.1.1.5 Evaluation of Embankment Integrity. For earth-filled dams, resistivity testing can be used to assess the integrity of the embankment materials. Variations in resistivity within the embankment can indicate areas of potential weakness or saturation, which may require remedial actions such as compaction or drainage improvements.

1.1.2 Background Study of 2D-Electrical Resistivity Tomography (ERT)

1.1.2.1 Historical Development. ERT has its roots in electrical prospecting methods developed in early 20th century. The application of resistivity measurements to map subsurface structures gained prominence in the mid-20th century with development of more sophisticated instrumentation and computational techniques. Transition from manual resistivity profiling to tomographic imaging occurred in late 20th century, driven by advancements in computer technology and mathematical algorithms.

1.1.2.2 Principle. ERT has its roots in electrical prospecting methods developed in the early 20th century. The application of resistivity measurements to map subsurface structures gained prominence in the mid-20th century with the development of more sophisticated instrumentation and computational techniques. The transition from manual resistivity profiling to tomographic imaging occurred in the late 20th century, driven by advancements in computer technology and mathematical algorithms.

1.1.2.3 Instrumentation. Modern ERT systems consist of a control unit, electrodes, cables, and a computer for data acquisition and processing. Electrical current is introduced into the ground through a pair of electrodes, while potential differences are measured using another pair of electrodes. Multiple configurations, such as Wenner, dipole-dipole, or pole-dipole, can be used depending on the desired depth of investigation and resolution.

1.1.2.4 Applications. ERT has diverse applications in geophysics, environmental studies, archaeology, civil engineering, and hydrogeology. In civil engineering, ERT is used for site characterization, subsurface imaging for infrastructure projects, and monitoring of geotechnical structures such as dams and levees. In environmental studies, ERT helps in delineating contaminant plumes, mapping groundwater resources, and assessing the integrity of waste containment facilities. Archaeologists use ERT to identify buried artifacts, archaeological features, and ancient structures without disturbing the site.

1.1.2.5 Challenges and Limitations. Interpretation of ERT data can be complex due to factors such as electrode spacing, subsurface heterogeneity, and noise interference. Resolution decreases with depth, limiting the applicability of ERT to shallow subsurface investigations in some cases. Environmental conditions, such as soil moisture and temperature, can affect resistivity measurements, requiring careful calibration and quality control.

1.1.3 Methodology of Electro Tomography for Resistivity Testing

- **1.1.3.1 Electrodes Placement.** Electrodes are strategically placed around the dam to measure resistivity at various depths.
- **1.1.3.2 Data Collection.** Electric currents are injected into the dam, and the resulting voltage measurements are recorded.
- **1.1.3.3 Imaging Analysis.** Advanced algorithms create 2D or 3D images depicting the distribution of resistivity within the dam.

1.1.3 Using 2D Electrical Resistivity Tomography (ERT) offers several benefits across various fields of application

1.1.4.1 Non-invasive Imaging. 2D ERT allows for the visualization of subsurface structures without the need for invasive drilling or excavation. This non-destructive nature is particularly advantageous in archaeological studies, environmental assessments, and civil engineering projects where preserving the integrity of the site is crucial.

1.1.4.2 High Resolution. While 2D ERT may not provide the same level of detail as 3D methods, it still offers relatively high resolution for shallow subsurface investigations. It can delineate features with fine spatial resolution, making it suitable for mapping near-surface geological layers, detecting buried objects, and characterizing shallow hydrogeological conditions.

1.1.4.3 Cost-effective. Compared to more complex and expensive geophysical techniques like 3D ERT or seismic imaging, 2D ERT is often more cost-effective. Its simplicity in terms of data acquisition and processing makes it accessible to a wider range of researchers & engineers.

1.1.4.4 Rapid Data Collection. With modern instrumentation and software, data acquisition for 2D ERT surveys can be conducted relatively quickly, allowing for rapid field assessments and timely decision-making. This efficiency is valuable for time-sensitive applications such as site investigations for construction projects or emergency response efforts.

1.1.4.5 Versatility. 2D ERT can be applied across various environments and geologic settings. It is used in diverse fields including geotechnical engineering, groundwater exploration, environmental monitoring, archaeology, and mineral exploration. Its versatility makes it a valuable tool for addressing a wide range of subsurface imaging needs.

1.1.4.6 Quantitative Analysis. ERT data can be processed to provide quantitative information about subsurface properties such as electrical resistivity, which can be correlated with material properties like lithology, porosity, and moisture content. This quantitative analysis enhances the interpretation of subsurface structures and facilitates informed decision-making in engineering and environmental applications.

1.1.4.7 Integration with Other Techniques. 2D ERT can be integrated with other geophysical methods, such as ground-penetrating radar (GPR), seismic surveys, and borehole logging, to provide complementary information and improve subsurface characterization. By combining multiple techniques, researchers and practitioners can achieve a more comprehensive understanding of subsurface conditions.

1.1.4.8 Risk Mitigation. By identifying potential hazards such as buried utilities, unexploded ordnance, or geotechnical anomalies, 2D ERT surveys contribute to risk mitigation efforts in construction, infrastructure development, and environmental remediation projects. Early detection of subsurface features minimizes the likelihood of costly delays, accidents, or environmental damage during project implementation.

Overall, the benefits of using 2D ERT include its non-invasiveness, high resolution, costeffectiveness, versatility, quantitative analysis capabilities, compatibility with other techniques, and contribution to risk mitigation in various applications requiring subsurface imaging.

1.2 Problem Statement

Current methods for assessing the health and integrity of dam infrastructure rely on drilling, are invasive in nature and provide access to smaller depth. This study aims to address this gap by investigating the utilization of Non Destructive Geophysical techniques. Electrical Resistivity Tomography (ERT) in conjunction with drone surveys will be used to enhance the efficiency and depth of dam health evaluations. The primary concern revolves around the need for a more advanced and integrated approach to monitor and assess dam structures, identifying potential weaknesses and environmental impacts to ensure proactive maintenance and risk mitigation strategies.

1.3 Objectives

1.3.1 Objective 1

To identify leakage zones in Gul Dheri Dam using ERT Survey and Drone Survey

1.3.2 Objective 2

To map the leakage zones in the study area by using ArcGIS for calculating areas of saturated, partially saturated and unsaturated zones

1.3.4 Objective 4

To recommend remedial measures for enhancing the life of a dam

1.4 Organization of the Report

The thesis begins with Chapter 1 discussing the major concept of Geophysical Survey (Electric Resistivity Tomography) in non-destructive Health assessment of Dam. Chapter 2 builds upon the general outline of the thesis, giving a deeper insight into the literature review conducted to support the thesis. Moving to Chapter 3, the thesis explains the methodology used in the research and interpretation of data obtained through ERT Survey followed by Drone Survey of Gul Dheri Dam. Chapter 4 gives the details about the remedial measures required to address leakge issues in dams.

CHAPTER 2

LITERATURE REVIEW

2.1 Introduction

The safe operation and proper maintenance of dam infrastructure is critical taking into account social and economic impacts in case of a failure. Currently, the assessment of dam structures is based on geodetic and geotechnical monitoring instrumentation. However, the majority of these instruments do not provide repeatable and reliable information about the mechanisms occurring inside the body of dams that could compromise the integrity of the structure. Geophysical methods can provide important information to define the safety level of dam infrastructure. Continuous dam monitoring would also enable early remedial maintenance and repair actions to be carried out improving public safety and reducing costs for dam owners, insurers and maintainers. The maintenance and inspection of dams are therefore of increasing importance. Systematic monitoring of dam infrastructure can deliver key information providing early warning signs of an impending failure and a better understanding of the ongoing performance of the structure.

2.2 Dam Health Assessment

Health and structural assessment of dams is imperative due to the critical role these structures play in managing water resources, providing hydroelectric power, and safeguarding downstream communities. Dams are complex engineering marvels that are subjected to various environmental forces, such as water pressure, seismic activity, and erosion, which can impact their structural integrity over time. Regular health assessments help identify potential vulnerabilities, enabling proactive maintenance and repairs to prevent catastrophic failures. A comprehensive evaluation of a dam's health involves monitoring factors like seepage, deformation, and concrete degradation. Additionally, technological advancements, such as remote sensing and sensor networks, contribute to more accurate and timely assessments, ensuring the safety and reliability of dams in the face of evolving environmental conditions.

Moreover, with the increasing impact of climate change, understanding how dams respond to extreme weather events is crucial for designing resilient structures that can withstand the challenges posed by a changing environment. Overall, health and structural assessments of dams are indispensable for mitigating risks, ensuring public safety, and optimizing the longterm performance of these critical infrastructure assets.

In many occasions internal erosion and seepage mechanisms within the body of dams are very difficult to be detected by conventional methods. The current practice to assess these mechanisms is based on geotechnical instrumentation. However, major issues exist with geotechnical instruments as they do not provide distributed, repeatable and reliable information in the long-term due to lack of 3rd Joint International Symposium on Deformation Monitoring Vienna, 30th March - 1st April 2016 maintenance as they are located inside the body of the dam. Geodetic measurements are also used to assess the deformations of the structure but they do not provide a direct insight of the internal mechanisms that influence the dam behavior. The assessment and safety of dam infrastructure would be significantly improved with non-destructive techniques that would allow internal erosion and seepage characteristics to be detected. [1]

2.3 Teton Dam Disaster 1976

One notable example of a dam disaster attributed to seepage is the failure of the Teton Dam in Idaho, USA, in 1976. The Teton Dam was an earthen dam constructed on the Teton River for irrigation and flood control. The disaster occurred on June 5, 1976, just a few months after the dam's completion. The primary cause of the failure was identified as seepage through the dam's foundation and abutments.

During the initial filling of the reservoir, seepage was observed at the base of the dam. Despite efforts to address the seepage, it intensified, leading to the disintegration of the dam structure. Ultimately, the dam breached, releasing an immense volume of water downstream. The catastrophic failure resulted in the loss of 11 lives, the destruction of numerous homes, and significant damage to agricultural land and infrastructure.

The Teton Dam failure highlighted the critical importance of thorough geotechnical investigations, proper foundation preparation, and vigilant monitoring for signs of seepage in dam construction. Subsequent dam engineering practices have incorporated lessons learned from such disasters to enhance the safety and reliability of dams worldwide.



Figure 1:Teton Dam Failure

2.4 Various Techniques of Dam Health Assessment

Assessing the health of dams involves employing various techniques to monitor and evaluate structural integrity, seepage, and other critical parameters. Some common techniques include

2.4.1 Geotechnical Investigations.

Detailed analysis of soil and rock properties at the dam site helps engineers understand the foundation's stability and potential risks.

2.4.2 Instrumentation and Monitoring.

Installation of sensors and monitoring equipment, such as piezometers, tiltmeters, and strain gauges, to continuously track factors like water pressure, movement, and stress within the dam structure.

2.4.3 Seismic Analysis.

Evaluating the dam's resilience to seismic activity by conducting seismic hazard assessments and implementing measures to enhance seismic resistance.

2.4.4 Non-Destructive Testing (NDT).

Employing methods like ultrasonic testing, ground-penetrating radar, and infrared thermography to assess the condition of dam materials without causing damage.

2.5 Electric Resistivity Tomography

Electrical Resistivity Tomography (ERT) stands as a powerful geophysical method employed in assessing the health of dams and other civil structures. By mapping the subsurface resistivity distribution, ERT provides valuable insights into the integrity of dam foundations, detecting potential seepage paths, voids, and structural weaknesses. This non-invasive technique plays a pivotal role in ensuring the safety and reliability of dams.

2.5.1 Procedure

The ERT procedure involves a systematic approach to gather resistivity data across the dam and its surroundings

2.5.1.1 Array Setup. Electrodes are strategically placed in an array along the dam and its vicinity. Arrays can vary, with common configurations being Wenner, Schlumberger, or dipole-dipole, depending on the specific goals of the assessment.

2.5.1.2 Current Injection. A known electrical current is injected into the ground through two electrodes, while potential differences are measured using other electrodes. The resulting voltage distribution allows the determination of subsurface resistivity.

2.5.1.3 Data Acquisition. Resistivity measurements are taken at multiple electrode positions to create a resistivity profile. These profiles are then combined to generate a 2D or 3D image of the subsurface, depicting variations in resistivity.

2.5.1.4 Data Interpretation. Interpretation involves identifying anomalies in the resistivity distribution, such as areas of higher or lower resistivity, which may indicate the presence of geological features or structural issues.

2.5.2 Benefits.

ERT offers several advantages in dam health assessments. These include

2.5.2.1 Subsurface Imaging. ERT provides detailed images of the subsurface, allowing engineers to identify potential issues, such as seepage paths, voids, or variations in material properties.

2.5.2.2 Non-Destructive. As a non-invasive method, ERT minimizes the impact on the dam structure, making it a preferred choice for routine health assessments without compromising dam integrity.

2.5.2.3 Cost-Effective. Compared to traditional drilling methods, ERT can be a cost-effective solution, especially when dealing with large dam structures or challenging terrains.

2.5.2.4 Real-Time Monitoring. ERT allows for real-time monitoring, enabling continuous assessments and timely detection of changes in subsurface conditions (Karaoulis, 2018)

2.6 Surveying and Geographic information System (GIS): Exploring the capabilities of Unmanned Aerial Vehicle (UAV/Drones)

2.6.1 General

Drone surveys can be used for various purposes, including; dams and bridges inspections [2] land surveying [3]; construction monitoring [4], slope monitoring [5] and urban traffic monitoring [6]

A desktop application for GIS drone mapping such as ArcGIS Drone2Map software has been used to capture drone data using site scan flight or any third party drone data collection app (ESRI, 2022). After the flight, the user creates a project to download the drone imagery. Ground control points are added and 2D and 3D outputs are processed and created. With the 2D or 3D outputs, measurements can be made, changes tracked, and other analysis can be made. Advanced analysis can also be done in ArcGIS Pro, utilizing a range of tools such as advanced spatial, temporal and spectral analysis, as well as data management (ESRI, 2022).

2.6.2 Benefits of Drone Survey

2.6.2.1 Improved Quality and Accuracy. Drones are essential due to their ability to capture reality, that is, images with coordinates and elevation, thereby ensuring quality through capturing many images and the use of sensors such LIDAR, infrared cameras and multi-spectral sensors (MRICS, 2019). Software such as Pix4D allows users to add ground control points (GCPs) to help improve data accuracy.

2.6.2.2 Saving Time. Given that drones make it easier to convey data in real-time, while automation of data processing and export is also made easier (ESRI, 2022) by software such as DroneDeploy, Pix4D, and thereby allowing users to save time and costs.

2.6.2.3 Improved Work Safety and Coverage. Given that drones are controlled remotely, they are safer to use, and have the ability to reach inaccessible areas, including areas dangerous to human life.

2.6.2.4 Integration with other GIS Software. Image processing software is part of GIS system, which provides access to tools for geospatial analysis. As such, end products from drone software can be exported and opened easily in ArcGIS desktop or ArcGIS Pro for integrated management. ArcGIS Online or ArcGIS Enterprise can also be utilized to publish or share the imagery data products and their elevation datasets (Seo et al. 2018).

2.6.3 Methodology

The study consists of five stages (Figure 1) which are: pre-survey, drone preparation, data acquisition, image processing and further processing with ArcGIS.

2.6.4 Integration with GIS

The DSM model generated by Pix4D was exported into ArcGIS desktop 10.8, creating contour lines map, providing the user with better understanding of the surface of the area captured by the drone. By exporting the DSM into ArcGIS, contours with elevation of 10m and resolution 100cm were generated.

2.7 Summary

Ensuring the safety and maintenance of dams is vital, considering their profound social and economic consequences in the event of failure. While current monitoring techniques lack reliability in detecting internal structural risks, integrating geophysical methods can offer early warnings, facilitate prompt remedial actions, and reduce costs, underlining the importance of systematic monitoring and inspection protocols for dam infrastructure.

The failure of the Teton Dam in 1976, attributed to seepage through its foundation and abutments during initial reservoir filling, resulted in catastrophic consequences including loss of life and extensive damage. This disaster underscored the necessity of rigorous geotechnical investigations, adequate foundation preparation, and vigilant monitoring to detect and address seepage in dam construction, influencing subsequent engineering practices globally.

Electrical Resistivity Tomography (ERT) is a powerful geophysical method used to assess the health of dams and civil structures by mapping subsurface resistivity, providing insights into dam foundation integrity, identifying seepage paths, voids, and structural weaknesses, thus playing a crucial role in ensuring dam safety and reliability. Drone surveys, offer versatile applications such as inspections of dams and bridges, land surveying, construction monitoring,

slope monitoring, and urban traffic monitoring, enabling efficient data collection, analysis, and visualization for various infrastructure projects.

CHAPTER 3

METHODOLOGY AND FIELD TESTS / RESULTS



3.1 ERT Survey

3.1.1 General

Leakage or fluid-filled fractures' response to electrical current is a very remarkable signature and provides relatively lower electrical resistivity values as compared to the surrounding host rocks. Therefore, this makes electrical resistivity tomography an excellent technique in exploration. Additionally, it is also one of the challenging tasks to provide probable location and direction of seepage flow or trends in the study area. In this project, electrical resistivity tomography profiling, topographic and surface geological map data acquired through drone survey will be integrated to get meaningful results to mark probable location and direction of dam leakage in the studied area. The electrical resistivity method is used in the study of horizontal and vertical discontinuities and uses the electrical properties of the ground. It utilizes direct currents or low-frequency alternating currents to investigate the electrical properties (resistivity) of the subsurface.

The age of the rock is an important factor of resistivity as a young volcanic rock (Quaternary Rocks) shows lower resistivity compared with old volcanic rock (Precambrian Rocks). Electrical resistivity is commonly applied to groundwater exploration, mineral exploration, detection of cavities and Oil exploration.

3.1.2 Resisitivity Imaging Equipment

The Geomative GD-10 (Resistivity Meter /IP) was utilized to acquire electrical resistivity data in the research area. This equipment is commonly used for groundwater exploration, environmental and engineering and geological investigations, etc. it is light in weight, convenient in operation, having a transmitter and receiver combined in the same case.

The nine configurations of electrodes are displayed by large LCD. Other main accessories of the equipment are a 300 m long multi-electrode intelligent cable with 5 m takeout spacing, four rechargeable batteries, 60 stainless steel electrodes, hammers, and measuring tapes. The Geomative GD-10 resistivity / IP meter is designed for optimum versatility in infrastructure projects and environmental studies. Additionally, the tool has built-in quality control and feedback to the operator. It can acquire 1D, 2D and 3D, and 4D geoelectrical resistivity surveys.

3.1.3 Fieldwork / Results

An ERT survey was applied on Gul Dheri Dam in Nowshera for delineating potential seepage paths contributing to the reservoir storage loss. By Interpreting the acquired geophysical results by different interpretative techniques, such as the 2D inversion of ERT, practiced on 6 executed profiles (ERT-P1, ERT-P2, ERT-P3, ERT-P4, ERT-P5 and ERT-P6).

This work aimed to identify probable dam leakage pathways in the dam site. The GPS-based topography was used for electrode location along with the ERT profiles and each profile having a length of 300 meters and starting (1st) electrode & ending (60th) electrode and Midpoint (30th) electrode.



Figure 2: ERT Profiles

3.1.3.1 ERT P-1



Figure 3:ERT P-1

ERT-P1 was acquired in the Down Stream area (as shown in Figure 5). The profile was orientated in the East-West direction, the midpoint of the ERT-P1 profile was lined up as per

the geotechnical diagram especially the installed piezometer borehole. The maximum profile length was 300 meters with a 5-meter takeout interval. The starting point (1st electrode) was placed in the eastern part and extended toward the western part. All electrodes were installed in the natural subsurface ground and planted electrodes were wetted with salty water to ensure good contact with the underlying soil and to reduce the noise. Figure 4 shows the inverted ERT-P1 geo-electrical cross-section (COVERING EAST-WEST NALA SECTION & SEGMENT OF SPILLWAY), which can be divided into three principal distinguished blocks horizontally.



- 2. Stream Deposit (Partly Saturated)
- 3. Spillway & Abutments
- 4. Fully Saturated Condition beneath Spillway, Left Abutment & Front of Dam Body
- 5. Active Seepage zones

Figure 4: ERT P-1 (2D-ERT)



Legends:

- 1. Slate (Unsaturated / Partly Fractured / Competent Bedrock)
- 2. Stream Deposit (Partly Saturated)
- 3. Spillway & Abutments
- 4. Fully Saturated Condition beneath Spillway, Left Abutment & Front of Dam Body
- 5. Active Seepage zones

Figure 5: ERT P-1 (2D-ERT)

Figures illustrate the ERT-P1 (downstream) electrical resistivity cross-section, all major features were incorporated in the figure such as left and right abutment, spillway, unsaturated, partly saturated, saturated zone, and active seepage zone.

The 1st zone is directly related to the HIGHLY SATURATED ZONE, marked clearly by blue color as shown in Fiure. The electrical resistivity of this HIGHLY SATURATED ZONE is between 20 to 60 ohm-m and which is started at 500 m (MSL) and extended up to 420 m (MSL), contrary meaning is that the depth of this HIGHLY SATURATED ZONE is started below 20 m depth (below the left abutment). This conductive zone is subsurface water flow channels. Left abutment comprise of rocks that are highly fragile in nature and provide passage to reservoir water which ultimately decreases the overall electrical resistivity of this zone.

The 2nd zone is MODERATE RESISTIVE ZONE, which is clearly shown by green color as shown in Figure. The electrical resistivity of this MODERATE RESISTIVE ZONE is between 60 to 170 ohm-m and starts from 490 m (MSL) below and extended up to 460 m (MSL) as shown in Figure. Field condition shows that downstream side comprises rounded to sub rounded gravels, pebbles, cobbles, sand,. The stream water is gradually infiltrated in friable sand & sandstone formation. The electrical resistivity cross-section shows overall resistivity of the formation is reduced due to the percolation of surface water into the friable sandstone formation. The cumulative resistivity of the sandstone formation is up to 170 Ohm-m.

The 3rd zone is a HIGH RESISTIVE ZONE, which is clearly shown by red color as shown in Figure. The electrical resistivity of this HIGH RESISTIVE ZONE is greater than 180 ohm-m and starts from 490 m (MSL) and extended up to 440 m (MSL) toward right abutment. High resistivity values are interpreted as unsaturated / slightly fractured formation as justified by field condition. The right abutment comprises of moderate thickness of slate rock, however, the left abutment slate formation is very fractured, thin bedded and weaker which probably help in infiltration of reservoir water.

3.1.3.2 ERT P-2



Figure 6: ERT P-2

ERT-P2 was acquired on the Dam Axis (as shown in Figure). The profile was orientated in the East-West direction. The maximum profile length was 300 meters with a 5-meter takeout interval. The starting point (1st electrode) was placed in the eastern part (Left) and extended toward the Western part (Right Abutment); this profile provides the East-West cross-sectional area of the dam axis and small portion of spillway and right abutment. All electrodes were installed in the natural subsurface ground and planted electrodes were wetted with salty water to ensure good contact with the underlying soil and to reduce the noise.



Legends:

1. Slate (Unsaturated / Partly Fractured / Competent Bedrock)

2.

3. Spillway & Abutments

4. Dam body

5. Clay & Silt Materials



Figure 7: ERT P-2 (2D-ERT)

Figure 8: ERT P-2 (2D-ERT)

Figures Illustrate the ERT-P2 (DAM AXIS) electrical resistivity cross-section, all major features were incorporated.

The 1st zone is HIGHLY SATURATED ZONE directly related to the clayey core of the dam body, marked clearly by blue color as shown in Figure. The junction of green yellow (below blue color) represents the lower boundaries of the clay core. The electrical resistivity of the clayey core zone is 20 to 60 ohm-m. The clayey core of the dam seems to be intact and structurally homogenous and lower boundaries are clearly distinct especially at the contact with the underlying bedrock. It is important to note that abnormal conductive zone is detected at deeper depth in the central part of the dam body. It is suggested to mark more analysis to identify the problem. Furthermore, spillway was constructed on highly fractured, thin bedded and incompetent slate rock portion of formation, the ERT P2 result indicates very conductive zone below the spillway which is interpreted as active seepage zone beneath the spillway and left abutment.

The 2nd zone is MODERATE RESISTIVE ZONE, which is clearly shown by green color as shown in Figure. The electrical resistivity of this MODERATE RESISTIVE ZONE is between 50 to 150 ohm-m and starts from 500 m (MSL) below and extended up to 490 m (MSL) as shown in Figure.

The 3rd zone is a HIGH RESISTIVE ZONE. The electrical resistivity of this HIGH RESISTIVE ZONE is greater than 170 ohm-m and starts from 490 m (MSL) and extended up to 440 m (MSL) toward the both abutments and top portion of the High resistivity values are interpreted as unsaturated slate rock formation as justified by field condition.



3.1.3.4 ERT P-3

Figure 9: ERT P-3

ERT-P3 was acquired which covers the Upstream Toe. The profile was orientated in the East-West direction. The maximum profile length was 300 meters with a 5-meter takeout interval. The starting point (1st electrode) was placed in the eastern part and extended toward the western part (Right Abutment). All electrodes were installed in the natural subsurface ground

and planted electrodes were wetted with salty water to ensure good contact with the underlying soil and to reduce the noise.



5. Probably link to Active Seepage zones in down stream area

Figure 10: ERT P-3 (2D-ERT)



Spillway & Abutments

4. Fully Saturated Condition in Dam Reservoir

5. Probably link to Active Seepage zones in down stream area

Figure 11: ERT P-3 (2D-ERT)

Figures Illustrate the ERT-P3 (UPSTREAM TOE) electrical resistivity crosssection, all major features were incorporated.

The 1st zone is directly related to the HIGHLY SATURATED ZONE, marked clearly by blue color. The electrical resistivity of this HIGHLY SATURATED ZONE is between 20 to 60 ohm-m and which is located elevation ranges 490~450 m (MSL), in other words, it means that

the depth of this HIGHLY SATURATED ZONE is between 10~40 m depth. The surface geological condition indicates that the right abutment is comprised of moderate thickness of slate rock, however, the left abutment slate rock is highly fractured and thin bedded. By incorporating the geological information, electrical resistivity tomography results revealed that highly fractured and thin bedded slate rock are mainly responsible for percolating and infiltrating the surface stream's water into the subsurface groundwater system. This formation possess high porosity and permeability which allow to saturate clay stone / mudstone and ultimately produced low electrical resistivity in this zone.

The 2nd zone is MODERATE RESISTIVE ZONE, which is clearly shown by green color. The electrical resistivity of this MODERATE RESISTIVE ZONE is between 60 to 170 ohm-m and which starts from 510 m (MSL) and extended up to 480 m (MSL), contrary meaning is that the depth of this MODERATE RESISTIVE ZONE occupies almost half portion in the cross-section. As the surface geological condition depicts that alluvial deposit responsible for reducing the electrical resistivity of the formation by allowing surface water to infiltrate. Hence, reduced the overall resistivity of the formation up to 170 Ohm-m. The surface geological condition indicates that the right abutment is comprised of moderate bedded slate rock formation.

The 3rd zone is a HIGH RESISTIVE ZONE, which is clearly shown by red color. The electrical resistivity of this HIGH RESISTIVE ZONE is greater than 170 ohm-m and starts from 520 m (MSL) and extended up to 480 m (MSL) toward the both abutments. High resistivity values are interpreted as unsaturated slate rock as justified by field condition.

3.1.3.4 ERT P-4



Figure 12: ERT P-4

ERT-P4 was acquired in RIGHT ABUTMENT. The profile was orientated in the North-South direction. The maximum profile length was 300 meters with a 5-meter takeout interval. The starting point (1st electrode) was placed upstream and extended toward downstream area (this profile was acquired along the road side). All electrodes were installed in the natural subsurface ground and planted electrodes were wetted with salty water to ensure good contact with the underlying soil and to reduce the noise.



- 6. Reservoir Area
- 7. Down Stream Area (Conductive anomalies need further investigation)





Figure 14: ERT P-4 (2D ERT)

Figures Illustrate the ERT-P4 (RIGHT ABUTMENT) electrical resistivity crosssection), all major features were incorporated.

The 1st zone is directly related to the HIGHLY SATURATED ZONE, marked clearly by blue color. The electrical resistivity of this HIGHLY SATURATED ZONE is between 20 to 60 ohm-m. The conductive anomalous zones in ERT-P4 profile can be further divided into two zone. There are two conductive anomalies present in the subsurface, one is located in upstream

area and other is in downstream area, the downstream conductive anomaly is widely distributed along the road side.

(GREEN COLOR) The 2nd zone of the ERT-P4 section shows partly saturated slate formation with moderate electrical resistivity values. Surface geological observation that that the right abutment is also composed of slate formation. The 2nd zone is MODERATE RESISTIVE ZONE, which is clearly shown by green color as shown in Figure 7. The electrical resistivity of this MODERATE RESISTIVE ZONE is between 60 to 170 ohm-m. The 3rd zone is a HIGH RESISTIVE ZONE, which is clearly shown by red color as shown in Figure 7. The electrical resistivity of this HIGH RESISTIVE ZONE is greater than 170 ohm-m.

3.1.3.5 ERT P-5



Figure 15: ERT P-5

ERT-P5 was acquired in LEFT ABUTMENT. The profile was orientated in the North-South direction, the midpoint of the ERT-P5 profile was lined up as per the geotechnical diagram (midpoint of ERT-P5 was perpendicular to dam crosssection). The maximum profile length was 300 meters with a 5-meter takeout interval. The starting point (1st electrode) was placed upstream and extended toward downstream area (this profile was acquired along the road side). All electrodes were installed in the natural subsurface ground and planted electrodes were wetted with salty water to ensure good contact with the underlying soil and to reduce the noise.



- 6. Reservoir Area
- 7. Down Stream Area

Figures illustrate the ERT-P5 (LEFT ABUTMENT) electrical resistivity cross-section, all major features were incorporated in the figure.

The 1st zone is directly related to the HIGHLY SATURATED ZONE, marked clearly by blue color. The electrical resistivity of this HIGHLY SATURATED ZONE is between 20 to 60 ohm-m. There is one conductive anomaly has been observed in downstream area, which justifies the leakage from the reservoir body.

(GREEN COLOR) The 2nd zone of the ERT-P5 section shows partly saturated deposit and slate formation with moderate electrical resistivity values. Surface geological observation reveal the right abutment is also composed of slate formation. The 2nd zone is MODERATE RESISTIVE ZONE, which is clearly shown by green color. The electrical resistivity of this MODERATE RESISTIVE ZONE is between 60 to 170 ohm-m.

The 3rd zone is a HIGH RESISTIVE ZONE, which is clearly shown by red color as. The electrical resistivity of this HIGH RESISTIVE ZONE is greater than 170 ohm-m.

3.1.3.6 ERT P-6

Figure 18: ERT P-6

ERT-P6 was acquired in LEFT ABUTMENT. The profile was orientated in the North-South direction. The maximum profile length was 300 meters with a 5-meter takeout interval. The starting point (1st electrode) was placed upstream and extended toward downstream area (this profile was acquired on the left abutment. All electrodes were installed in the natural subsurface ground and planted electrodes were wetted with salty water to ensure good contact with the underlying soil and to reduce the noise.

- 2. Stream Deposit (Partly Saturated)
- 3. Mid Point of Left Abutment / GD 10
- 4. Fully Saturated Condition in Dam Reservoir
- 5. Active Seepage zones (tie point on left abutment)
- 6. Reservoir Area
- 7. Down Stream Area

Figure 20: ERT P-6 (2D ERT)

Figures Illustrate the ERT-P6 (LEFT ABUTMENT) electrical resistivity crosssection, all major features were incorporated in the figure.

The 1st zone is directly related to the HIGHLY SATURATED ZONE, marked clearly by blue color. The electrical resistivity of this HIGHLY SATURATED ZONE is between 20 to 60 ohm-m. The first 30-meter depth in upstream area at 125-meter horizontal distance comprise of unsaturated condition in the upstream area, additionally, a conductive zone can be seen just below 80-meter horizontal distance at depth of 40 meter from NSL. In term of, elevation (m)

from MSL, the conductive at horizontal distance 80-meter at the elevation level below 480 m MSL. The upper part of this anomaly can be interpreted as clay core, however, the lower part at 480~490 m MSL are considered as the active seepage zones.

(GREEN COLOR) The 2nd zone of the ERT-P6 section shows partly saturated slate formation with moderate electrical resistivity values. Surface geological observation and geotechnical drilling reveal that the left abutment is also composed of slate formation.

The 2nd zone is MODERATE RESISTIVE ZONE, which is clearly shown by green color. The electrical resistivity of this MODERATE RESISTIVE ZONE is between 60 to 170 ohm-m. When the geological materials are partly saturated then these materials show moderate electrical resistivity values, when fractures are fully saturated then same materials show low electrical resistivity values.

The 3rd zone is a HIGH RESISTIVE ZONE, which is clearly shown by red color. The electrical resistivity of this HIGH RESISTIVE ZONE is greater than 170 ohm-m.

3.1.4 Areas of zones in ERT Profile using ArcGIS

Only the procedure for ERT P-2 (Dam Axis) is shown below

	Sime	ill v	6 X 0						
			* Gen	allowing Day, Georgeo	and the second		The second	100	Contraction of
			1 mg	All strength				A STATE	
	10		A. A.		· · ·	1		1 A 10 10 10 10	a second second
TT Image: P22_22022022 Image: P22_2202202 Image: P22_220220 Image: P22_220220 <thimage: p22_220220<="" th=""></thimage:>	- 6	C 22	THE R. LANSING	Site This	100				100.00
Control		Concession in which the			6			-	
Image: State		-	(1)			191			Statistics of
The image is a statistic in				all and the second				and some of	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Data Data <th< td=""><td></td><td></td><td>1.1</td><td></td><td>0</td><td></td><td>and the second s</td><td></td><td></td></th<>			1.1		0		and the second s		
Image: Example 2.2.2.00.00.02 Image: Example 2.2.2.00.00.02 Image: Image: <th< td=""><td></td><td></td><td></td><td>Sec. 1</td><td>• ·</td><td></td><td>and the second second</td><td></td><td></td></th<>				Sec. 1	• ·		and the second second		
Integr 12.22.20032002 1.4 23.8290544 4.4 <td></td> <td></td> <td>1 Color</td> <td>Market .</td> <td></td> <td>and the second</td> <td></td> <td>12 and the second</td> <td></td>			1 Color	Market .		and the second		12 and the second	
Construint Constr				M AND			Contra and	ALL DOUGHT OF	Statement of the local division of the local
TTT Image: State Proj Next State Proj							 University Constraints 	ENC 41.8292044 1 * 1.83	
New			1.91	1 2 2 1			The second se		
Normal Construction Normal Norman Normal Norman Normal Normal Normal Normal Norman Normal Norma	271	-13		103.3	72,89397	997E 33 SUBRISTIN 🐱		1.0	General General U. H
No. No. <td>211</td> <td></td> <td>- 33 + 191</td> <td>1.64</td> <td>72,00301</td> <td>99°E 33.3099517N 🐱</td> <td></td> <td>14</td> <td>General General ()</td>	211		- 33 + 191	1.64	72,00301	99°E 33.3099517N 🐱		14	General General ()
Long Names K Source V Mage K Mage	211 Mar: D	• Count	11 33 + 10 1 are X	1993	72888	974 33 30 BERT		14	General Generation 1
№ 1 44.21/2136 -TLABRAL #E04.024.01400 4.014.06.04.01100 0.00000 -L00000 0.00000 2 3 54.01/2000 -TLABRAL #E04.024.014000 4.014.01100 0.000000 -L000000 -L000000 0.000000 2 3 54.01/2100 -44.01/2100 64.01/2100 64.01/20000 0.0000000 0.000000	211 Mar: D	er, Gereri		III 1000ar Keyn	Parentes Nortices + [eore solataenspro. 🐱		14	(General Services II)
→ ↓ ¥X4.200200 +TL280498 ¥X04.84000 4.018.21.21.200 +D00000 +D000000 +D00000 +D000000 +D00000 +D000000 +D000000 +D00000 +D000000 +D000000 +D000000 +D000000 +D000000 +D000000 +D000000	ern Harr Di	er, Gorel B 4	11 23 + 10 1 tre X 17 % 14 12 = trex 1	I Ist Octaer Rolyn Secret V	7200081 Norius (MTHA) + [Mag 8	9978 33.3639953736 🐱	Repth of X	(-) Nambur 1	Foreine Territor (*) 11
2 36/23/15/s 44234/17/s 44243/12/set100 60/00000 60/0000	an Aar D A	er, Georef De Ar	10 25 + 10 1 are X 4 % 4 10 - hore I 60.7276	III Ist Octor Polyn Source V -TLampad	72.00381 007368 (MTINE) * [Mag X - 8.004.034.074800	Mag 1 Acristos antino	Reset of X 0.000000	Residue 1	F General Tensor () Residuel 0.000000
	an Aar Di Aar Di	int General Des A	III 23 + 40 1 III 23 + 40 1 III 24 No. 41 127 - 1 Second Statements	III Ist Octor Polyn Source V Taatanaa Taatanaa	Posses conservatives + Marc X 8.004.004000 8.002.0040000	Mag T Activities and activities Activities and activities	Restord X Galacted Galacted	Residue 1 -LOSCOD -LOSCOD	Resetual La Constanti II La Constanti Castanti II
Q 5 496/196460 -backweiter Backachte 4018/backweiter 0000000 -backweiter 0000000 Q 6 4018/backweit 115 /Plasee Backachte 4018/backweite 4018/backweite 0000000 -backweite 00000000 -backweite 0000000 -backweite 00000000 -backweite 00000000 -backweite 00000000 00000000 00000000 000000000 00000000 00000000		er, Georef Dek Lek J	III 23 + H) ang X III 24 + H) Alexandro Marcola Marcola Marcola Marcola Marcola	III]g Int Outer Roger Server V TLaternal TLaternal Security	728597 Conse (Million +) Mar X & Col 2214/2014/00 BCOL 2214/2014/00 BCOL 2214/2014/00	Mag T Antikation with Antikation with Antikation with	Restrict X Captrons Cattorne Cattorne	Residue 1 Accesso Accesso Accesso Accesso	Restui Dutocoo Datocoo Datocoo Datocoo
W 6 40000001 1000000 40000000 4000000 4000000<		er, Georef Den, Georef Den Lank	10 23 + 10 1 20 23 + 10 1 20 2 4 2 10 - 5000 1 60.212106 500.00100 500.00100	III III III Octor Roje Seret V -TLAINAL -TLAINAL -TLAINAL -TLAINAL -TLAINAL -TLAINAL	7235331 Conias (MTHer) * [Mage X BODA STRADMARCO BODA STRADMARCO BODA STRADMARCO BODA STRADMARCO BODA STRADMARCO	Miles V Administration w Administration Administration Administration	Residual X Galorado Galorado Galorado Galorado	Residue 1 -Locotor -Locotor Database Locotor	Residual Current Terrane (C) 11 Current Curren
P 3 380 (4802) -15 (918) 15 8.004/03 (9100) 4.018,301 (91100) 0.000000 0.00000 0.00000<		n General In General In An In An An In An In An	10 23 + 40 1 10 23 + 40 1 10 23 4 40 32 1 10 4	III Ist Oader Roger Searce V -T2.487643 -T2.587764 -T2.567764 -64.268764	Posses contai (VPInet *) Mar X BOG STROMARC BOG STROMARC BOG STROMARCO BOG STROMARCO BOG STROMARCO BOG STROMARCO	Miles II Activation processor Activation processor Activation processor Activation processor Activation processor Activation processor	Residual X 0.000000 0.000000 0.000000 0.000000 0.000000	C (1) Residue 1 - 4.000000 - 4.000000 - 4.000000 - 4.000000	Retricul Cutoros Cutor
No. Statistics Processing Reserved Addressing Concessing		r Geord In Geord Int Int Int Int Int Int Int Int Int Int	III 23 + H) IIII 23 + H) IIIII 23 + H) IIII 23 + H) IIII 23 + H) IIII 23 + H) IIII 23 + H) IIIII 23 + H) IIII 23 + H) IIII 23 + H) IIIII 23 + H) IIIII 23 + H) IIIII 23 + H) IIIIII 23 + H) IIIII 23 + H) IIIII 2	III]] Ist Octor Polyn Serrer V - TJ. Strong - TJ. Strong	PERSON Consult (Million +) Mary X BEDS STRACTURE BEDS STRACTURE BEDS STRACTURE BEDS STRACTURE BEDS STRACTURE BEDS STRACTURE	Miles V Activation and activation Activation and activation Activation and activation Activation and activation Activation activation Activation activation	Residuel X 0.000000 0.000000 0.000000 0.000000 0.000000	Residue # -Eccolori	Resetual Fernanes (C.) 11 Resetual Custosono Custos Cus
№ № 122.802162 -11.6666.00 8.004.12.2.666700 4.018.206000 0.000000 -2.00000 0.000000 № 10 162.80696 46.01.002.0000 4.018.206.0000 4.018.206.000 0.0000000		r Geord Des Constant Des Consta	10 23 + +0 1 mm X 4 4 4 50 - Second 642-2108 642-21	In III In Outer Roge Secret V TLANNAL TLANNA HALDRETT TLANTAL ALCONTON TS FOLSO TS FOLSO TS FOLSO TS FOLSO	ALIENSE COTAGE (MELLINE +) MARY X RECK STRACTING RECK STR	Map II 4016 33 32 36 37 37 37 40 4016 30 30 30 30 4016 31 32 39 30 4016 31 32 39 30 4016 31 32 39 30 4016 32 38 30 4016 32 38 30 4016 32 38 30 4016 32 38 30 4016 30 30 4016 30 30 4016 30 40 40 40 40 40 40 40 40 40 4	Reptivel X Currented Curre	Residue 1 -E.00000 -E.00000 B.00000 -L.00000 -L.00000 -L.00000 -L.00000 -L.00000 -L.00000 -L.00000	Resetual Termanes co. 11 Resetual Custoreto Custoreto Custoreto Custoreto Custoreto Custoreto Custoreto Custoreto Custoreto
NO TALANANG ALEXATION ALEXATION ALEXATION CODED DODDOD DODDOD <thdoddod< th=""> DODDOD <thdod< td=""><td>Arr D</td><td>n General In Automation In Aut</td><td></td><td>III III III Coder Roge Secret V TL carmad TL carmad TL carmad Carbon Carbon Carbon TL carmad Carbon</td><td>P200000 Contail (MT met *) Mar X B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Bok 2014/04/000</td><td>Miles V Acrimited Discovery All Acrimited Discovery A</td><td>Residual X 0.000000 0.000000 0.000000 0.000000 0.000000 0.000000 0.000000</td><td>Residue # -L.00000 -L.00000 L.00000 -L.00000 -L.00000 -L.00000 -L.00000</td><td>Residual Currento Cur</td></thdod<></thdoddod<>	Arr D	n General In Automation In Aut		III III III Coder Roge Secret V TL carmad TL carmad TL carmad Carbon Carbon Carbon TL carmad Carbon	P200000 Contail (MT met *) Mar X B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Dok 2014/04/00 B.Bok 2014/04/000	Miles V Acrimited Discovery All Acrimited Discovery A	Residual X 0.000000 0.000000 0.000000 0.000000 0.000000 0.000000 0.000000	Residue # -L.00000 -L.00000 L.00000 -L.00000 -L.00000 -L.00000 -L.00000	Residual Currento Cur
P 11 Martines - Transme annual later Annual Later annual annual annual		In Connect In Connect	III 2.0. + H) IIII 2.0. + H) IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	TLADING Secret V -TLADING -TLA	72.0000 contail (VPTinet *) Mar X 8.004.008.0000 8.004.008.000 8.004.000 8.004	Miles 1 4078-305-040-000 4078-306-040-000 4078-306-040-000 4078-306-040-000 4078-306-04000 4078-306-04000 4078-306-04000 4078-306-04000	Residual X 0.000000 0.0000000 0.0000000 0.0000000 0.000000	C 0 Rentral 7 - 2.00000 - 2.00000 - 2.00000 - 2.00000 - 2.00000 - 2.00000 - 2.00000 - 2.00000	Resolution Francesco C II Resolution Custopopopo Custopopo Custopopo Custopopo Custopopopopo Custopopopo Custopopopo Custopopo Custopopo Custopopo Custopopo Custopopo Custopopopo Custopopo Cus
The second		r General In General I J I J I J I J I J I J I J I J I J I J	10 23 + 40 1 10 23 + 40 1 10 24 12 10 10 24 10 10 1	Im Im Int Octor Polyn Secret V TLANDAL TL	Alexandre * (Mary X BODA IDEADANNO BODA IDEADANNO BODA IDEADANNO BODA IDEADANNO BODA IDEADANNO BODA IDEADANNO BODA IDEADANNO BODA IDEADANNO BODA IDEADANNO	Miles V Acrikulos (2001)	Resetuel X 0.200000 0.200000 0.200000 0.200000 0.200000 0.200000 0.200000 0.200000 0.200000 0.200000	Residue # -2.00000 -2.00000 -2.00000 -2.00000 -2.00000 -2.00000 -2.00000 -2.00000 -2.00000 -2.00000	Resetual Fernance of 1

Figure 21: Georeferencing 1

Figure 22: Georeferencing 2

Then supervised classification was done by using Support Vector Machine Classifier.

Figure 23: Classification 1

Figure 24: Classification 2

Areas were calculated by dividing each profile into 3 zones i.e. Saturated Zone, Partially Saturated Zone and Unsaturated Zone. The results are as shown below.

3.1.4.1 ERT P-1 (Downstream)

Figure 25: ERT P-1 (Classified)

Ser No	Zone	Color	Area(sqm)
1.	Saturated		1183.232854
2.	Partially Saturated		802.4545011
3.	Unsaturated		2837.343082
4.		Total	5857.706387

 Table 1: AREAS (ERT P-1)

3.1.4.2 ERT P-2 (Dam Axis)

Figure 26: ERT P-2 (Classified)

Ser No	Zone	Color	Area(sqm)
1.	Saturated		2127.914916
2.	Partially Saturated		971.0396796
3.	Unsaturated		3474.659957
4.		Total	6902.277892

 Table 2: AREAS (ERT P-2)

3.1.4.3 ERT P-3 (Upstream Toe)

Figure 27: ERT P-3 (Classified)

Ser No	Zone	Color	Area(sqm)
1.	Saturated		491.8345312
2.	Partially Saturated		294.284142
3.	Unsaturated		1802.85506
4		Total	2948.171789

Table 3: AREAS (ERT P-3)

3.1.4.4 ERT P-4 (Right Abutment)

Figure 28: ERT P-4 (Classified)

Ser No	Zone	Color	Area(sqm)
1.	Saturated		387.3952797
2.	Partially Saturated		284.3349064
3.	Unsaturated		5029.136916
4.	Total		6783.87297

Table 4: AREAS (ERT P-4)

3.1.4.5 ERT P-5 (Left Abutment 1 - Outer)

Figure 29: ERT P-5 (Classified)

Ser No	Zone	Color	Area(sqm)
1.	Saturated		333.4748811
2.	Partially Saturated		1154.041824
3.	Unsaturated		4200.499355
4.		Total	6021.881188

 Table 5: AREAS (ERT P-5)

3.1.4.6 ERT P-6 (Left Abutment 2 - Inner)

Figure 30: ERT P-6 (Classified)

Ser No	Zone	Color	Area(sqm)
1.	Saturated		673.47064
2.	Partially Saturated		1068.313103
3.	Unsaturated		6563.9973
4.		Total	9139.178173

 Table 6: AREAS (ERT P-6)

3.1.5 Dam Site View with zones of all profiles

Figure 31: Dam view with all profiles

3.2 Drone Survey

For better understanding of the terrain and its relationship with the seepage of water, a drone survey was carried out on Oct 31st, 2023.

Figure 32: Drone Survey

3.2.1 Benefits of Drone Survey

Following are some benefits for carrying out a drone survey.

3.2.1.1 Precision Monitoring. Drone surveys offer accurate, detailed data for monitoring earth fill dam conditions, detecting issues like erosion or structural deformities early on.

3.2.1.2 Time and Cost Efficiency. Drones swiftly cover large areas, reducing survey time and costs compared to traditional methods, making monitoring more accessible and affordable.

3.2.1.3 Enhanced Safety. By eliminating the need for personnel in hazardous terrains, drones improve safety during dam inspections, minimizing the risk of accidents or injuries.

3.2.1.4 Data Accessibility. Drone survey data is quickly processed, providing real-time insights. This accessibility allows for prompt decision-making and proactive maintenance to ensure dam stability.

3.2.2 Results of Drone Survey

Figure 33: Aerial View of Dam Site

Figure 34: Digital Elevation Model

The above model shows the elevation of the selected terrain on a heat map, with red being the highest and blue being the lowest / deepest areas.

Figure 35: Camera Locations

Above is a pictorial representation of the camera locations and the path it took while conducting the survey

Ser No	Heading	Remarks
1.	Number of images	840
2.	Flying altitude	133 m
3.	Ground resolution	2.92 cm/pix
4.	Coverage area	0.495 km ²
5.	Camera stations	838
6.	Tie points	2,229,832
7.	Projections	7,748,657
8.	Reprojection error	1.49 pix
9.	Elevation of area	470 – 580m above MSL

Fol are some specifications of the svy.

 Table 7: Drone Survey Specifications

CHAPTER 4

REMEDIAL MEASURES

-

SER NO	HEADING	DESCRIPTION	
1	Concenting of	Grouting involves injecting grout (a mixture of cement,	
1.	Grouting	water, and sometimes additives) into the cracks and fissures	
		in the slate formation to seal them and reduce permeability.	
		This can be done using various techniques such as	
		permeation grouting or pressure grouting.	
2		Constructing cutoff walls along the leakage zones can help	
2.	Cutoff walls	prevent water from seeping through the slate formation.	
		These walls are typically made of impermeable materials	
		such as concrete or steel sheet piles and are driven into the	
		ground to depths below the water table. These barriers can	
		be designed to extend to depths where the slate formation	
		transitions to less permeable strata.	
2	Garmanharman	Installing geomembranes on the upstream face of the dam	
5.	Geomemoranes	can act as a barrier to prevent water from penetrating	
		through the slate formation. Geomembranes are	
		impermeable liners made of synthetic materials such as	
		high-density polyethylene (HDPE) or polyvinyl chloride	
		(PVC).	
4	Dusingas	Implementing drainage systems within the dam structure	
4.	systems	can help collect and redirect water that seeps through the	
		slate formation away from the dam. This can involve	
		installing horizontal and vertical drains along with sumps	
		and pumps to manage water flow.	
E		Strengthening the dam structure through techniques such as	
5.	Keinforcement	soil stabilization or adding additional layers of fill material	
		can help reduce the potential for seepage through the slate	
		formation.	

		Regular monitoring of the dam and its foundation, including
6.	Monitoring and maintenance	instrumentation to measure water seepage and ground
		movement, can help identify potential issues early on.
		Routine maintenance activities such as inspecting grout
		curtains and repairing any damage promptly can help ensure
		the effectiveness of remedial measures over time.
7	Commontion	Compaction grouting involves injecting a low-slump, low-
7.	grouting	mobility grout into the soil or rock formation to densify and
	0 0	strengthen it. This technique can help reduce permeability
		by filling voids and stabilizing the surrounding ground.
0	Brossupo	In addition to traditional cement-based grouting, chemical
0.	grouting with	additives can be used to enhance the effectiveness of
	chemical	pressure grouting. These additives can improve the
	additives	penetration of grout into small fissures and provide better
		long-term sealing.
0	Vagatativa	Implementing vegetative covers on the downstream slope of
9.	Covers	the dam can help manage water infiltration by promoting
		evapotranspiration and reducing surface runoff. This natural
		approach can complement other remedial measures and
		provide additional stability to the dam embankment.
10	Floatnakinatia	Electrokinetic stabilization involves the application of an
10.	Stabilization	electric field to the ground to induce chemical reactions that
		alter the properties of the soil or rock. This method can help
		strengthen the slate formation and reduce permeability by
		promoting precipitation of minerals or by enhancing soil
		compaction.
11	Soil poiling	Soil nailing involves drilling steel bars or rods into the slope
11.	Son naming	or embankment and grouting them in place to reinforce the
		soil mass. This technique can stabilize the slope and reduce
		the potential for water infiltration through the slate
		formation.
10	Cail Dantsit-	Soil-bentonite cutoff walls utilize a mixture of soil and
12.	Soll-Bentonite Cutoff Walls	bentonite clay to create an impermeable barrier within the

	dam foundation. This technique can adapt to irregularities in	
	the slate formation and provide a flexible yet effective	
	solution for reducing leakage.	

Table 8: Remedial Measures

References

[1] Prakash, G. (2022, October). *Recent advancement of concrete dam health monitoring technology: A systematic literature review*. Retrieved from Sciencedirect: https://www.sciencedirect.com/science/article/pii/S2352012422006749

Seo, J., Duque, L., & Wacker, J., 2018. Drone-enabled bridge inspection methodology and application: Automation in Construction, Volume 94, Pages 112-126, ISSN 0926-5805.
 Available from: <u>https://doi.org/10.1016/j.autcon.2018.06.006</u>.

[3] MRICS, P.K. (2019). Drones: Applications and Compliance for Surveyors. Insight Paper. Available from: <u>https://www.MRICS.org/content/dam/MRICSglobal/documents/to-be-sorted/drones-applications-andcompliance-for-surveyors-MRICS.pdf</u>.

 [4] Naveed, A., Fawad, N., & Muhammad, A.I., 2018. Construction Monitoring and Reporting using Drones and Unmanned Aerial Vehicles (UAVs). The Tenth International Conference on Construction in the 21st Century (CITC-10) July 2nd -4th, 2018, Colombo, Sri

https://www.researchgate.net/publication/326264559_Construction_Monitoring_and_Repor ting_using_Drones_and_Unmanned_Aerial_Vehicles_UAVs/link/5b42f727aca2728a0d654 5ea/download]

 [5] Gómez, L.A., & Pabón, S.B., 2021. Using Drones and Remote Sensing for Slope Monitoring: a Review. Available from: https://api.semanticscholar.org/CorpusID:244113439.

[6] Butilă, E. V., & Răzvan, G. B., 2022. Urban Traffic Monitoring and Analysis Using Unmanned Aerial Vehicles (UAVs): A Systematic Literature Review: Remote Sensing 14, no. 3: 620. [https://doi.org/10.3390/rs14030620]. Cho, J., Lee, J. & Lee, B., 2022. Application of UAV Photogrammetry to Slope-Displacement Measurement. KSCE Journal of Civil Engineering 26, 1904–1913. Available from: https://doi.org/10.1007/s12205-021-1374-1.

turnitin

Digital Receipt

This receipt acknowledges that Turnitin received your paper. Below you will find the receipt information regarding your submission.

The first page of your submissions is displayed below.

Submission author:	Capt Irfan Haider Khan
Assignment title:	Quick Submit
Submission title:	COMBINE UAV AND 2D ERT APPROACH FOR IDENTIFICATION
File name:	Syn9_Irfan.docx
File size:	18.62M
Page count:	47
Word count:	7,328
Character count:	43,189
Submission date:	14-May-2024 01:54AM (UTC-0700)
Submission ID:	2379040241

	COMBINE UAV AND 2D ERT APPROACH FOR IDENTIFICATION OF WATER LEAKAGE ZONE IN GUL DHERI DAM, AKORA KHATTAK, NOSHEHRA
	FINAL YEAR PROJECT UG 2023
	By
	 Leader – CMS-356439 Capt Irfan Haider Khan Member 1 – CMS-356449 Capt Abdullah Khan Member 2 – CMS-356446 Capt Hanza Shahid Member 3 – CMS-356450 Capt Ejaz Akram
	Military College of Engineering, Risalpur Cantonment National University of Sciences and Technology, Islamabad, Pakistan
	YEAR 2023
Copyright 2024 Turnitin. All rig	ghts reserved.

COMBINE UAV AND 2D ERT APPROACH FOR IDENTIFICATION OF WATER LEAKAGE ZONE IN GUL DHERI DAM, AKORA KHATTAK, NOSHEHRA

ORIGINALITY REPORT					
1 SIMLA		7% INTERNET SOURCES	6% PUBLICATIONS	2% STUDENT A	APERS
PROVAR	riouxis				
1	Internet Sou	g.net			4%
2	W. Al-Fa leakage using e northw 2018 Publication	ares, J. Asfahani. e origin in Abu Ba lectrical resistivit estern Syria'', Ge	"Evaluation o aara earthen o ty tomography ofísica Intern	f the Jam /, acional,	3%
З	M.A. No Nwoko. the maj sustain Sustain Publication	wachukwu, L.I. N . "1D resistivity in pping of igneous able quarry deve able Mining, 201	lwosu, P.A. Uz nversion techr intrusives; A lopment", Jou 7	oije, C.A. nique in step to irnal of	1%
4	raee.ar	agon.es			1%
5	bibliote Internet Sou	kanauki.pl			<1%

6	Submitted to Queen's University of Belfast Student Paper	<1%
7	utilitiesone.com	<1%
8	Submitted to The University of the South Pacific Student Paper	<1%
9	Submitted to Mangosuthu University of Technology Student Paper	<1%
10	B. Schmitz, H. Kuhlmann, C. Holst. "Investigating the resolution capability of terrestrial laser scanners and its impact on the effective number of measurements", ISPRS Journal of Photogrammetry and Remote Sensing, 2020 Publication	<1%
11	Submitted to Learning Skills Partnership	<1%
12	geosyntheticsmagazine.com	<1%
13	washingtongreek.com	<1%
14	electrical-resistivity-tomography-pdf- 5322.peatix.com	<1%

15	www.geo-solutions.com	<1%
16	archive.researchdata.leeds.ac.uk	<1%
17	ancestralcoffee.com	<1%
18	www.mdeq.ms.gov	<1%
19	www.roodekloof.co.za	<1%
20	Mori, Guido <1978>(Ghirotti, Monica). "The use of Ground Penetrating Radar and alternative geophysical techniques for assessing embankments and dykes safety", Alma Mater Studiorum - Università di Bologna, 2011. Publication	<1%
21	stg-lamp.harker.org	<1%
22	www.researchgate.net	<1%
23	Panagiotis Michalis, Philippe Sentenac. "Subsurface condition assessment of critical dam infrastructure with non-invasive geophysical sensing", Environmental Earth Sciences, 2021	<1%

Publication

Exclude quotes On Exclude bibliography On Exclude matches Off